



FACULTY OF ENGINEERING
END OF SEMESTER EXAMINATIONS - APRIL 2025

PROGRAMME: BACHELOR IN CIVIL ENGINEERING

YEAR/SEM: YR. II/SEMESTER II

COURSE CODE: BCE 2202

COURSE NAME: STRUCTURAL ANALYSIS II

DATE: 15th/APRIL/2025

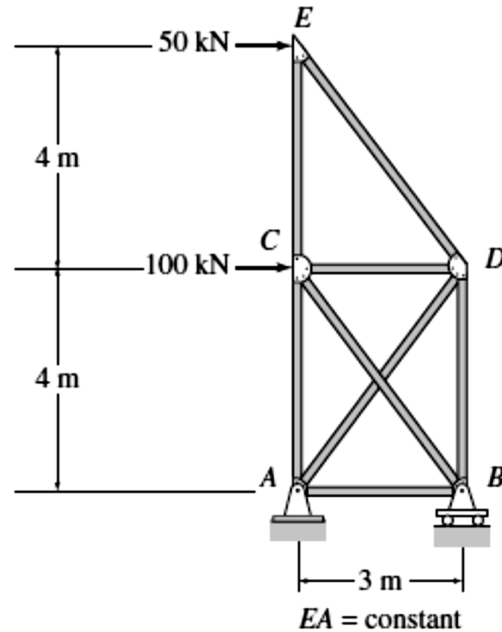
TIME: 9:00AM – 12:00PM

INSTRUCTIONS TO CANDIDATES:

- **THIS EXAMINATION PAPER CONSISTS OF SIX QUESTIONS.**
- **ATTEMPT ANY FOUR QUESTIONS FOR FULL MARKS**
- **DO NOT OPEN THIS EXAMINATION UNTIL YOU ARE TOLD TO DO SO**
- **ALL ROUGH WORK SHOULD BE IN YOUR ANSWER BOOKLET**
- **THE TIME ALLOWED FOR THIS EXAMINATION IS STRICTLY THREE HOURS**
- **ON THE FIRST PAGE OF YOUR ANSWER BOOKLET**
 - **WRITE YOUR REGISTRATION NUMBER PROPERLY**
 - **WRITE THE COURSE NAME AND COURSE CODE**
 - **WRITE EXAMINATION VENUE**
 - **DO NOT WRITE, DROW OR SCRATCH ANYTHING ELSE ON THE FIRST PAGE**
 - **WRITING UNNECESSARY INFORMATION LIKE PHONE NUMBERS IN THE FIRST PAGE SHALL ANNUL YOUR EXAM**
 - **ANSWER BOOKLETS THAT DO NOT CARRY THE REQUIRED INFORMATION, OR THAT HAVE UNNECESSARY WRITING IN THE FIRST PAGE SHALL NOT BE MARKED**

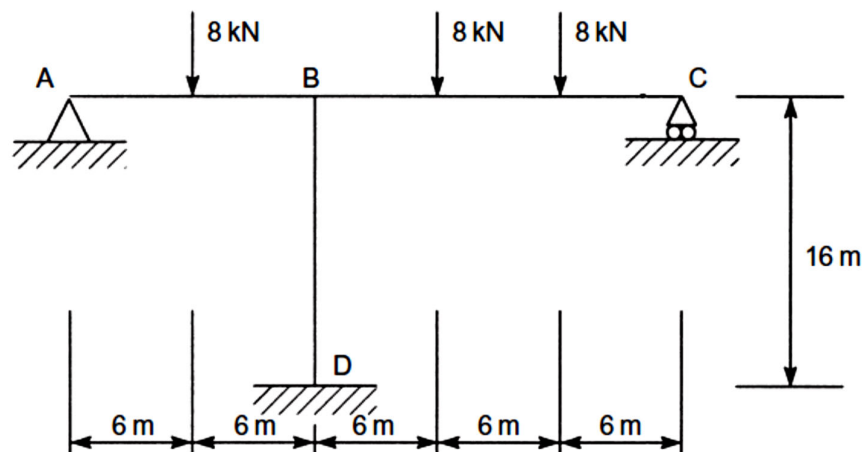
QUESTION ONE (25 Marks)

Determine the reactions and the force in each member of the truss shown in Fig. below using the method of consistent deformations - Force Method (25 marks)



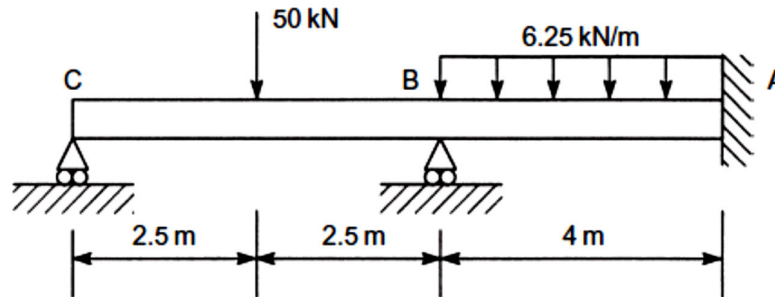
QUESTION TWO (25 Marks)

Calculate the end moments in the members of the frame shown in Fig. below using the Moment Distribution Method. The flexural rigidity of the members AB, BC and BD are $2EI$, $3EI$ and EI , respectively, and the support system is such that sway is prevented. (25 marks)



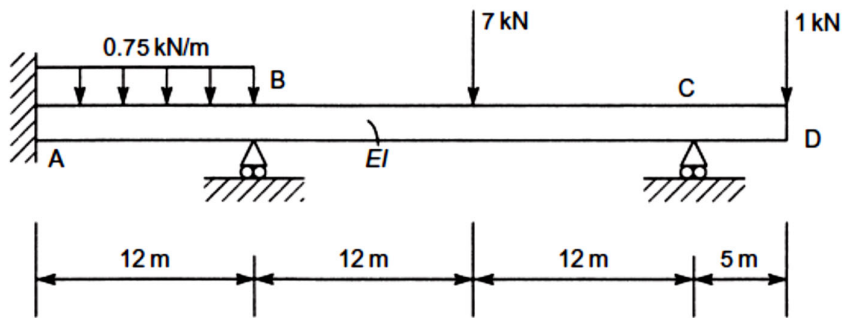
QUESTION THREE (25 Marks)

In the beam ABC shown in Fig. below the support at B settles by 10 mm when the loads are applied. If the second moment of area of the spans AB and BC are $83.4 \times 10^6 \text{ mm}^4$ and $125.1 \times 10^6 \text{ mm}^4$, respectively, and Young's modulus, E , of the material of the beam is 207000 N/mm^2 , calculate the support reactions using the Slope Deflection Method. **(25 marks)**



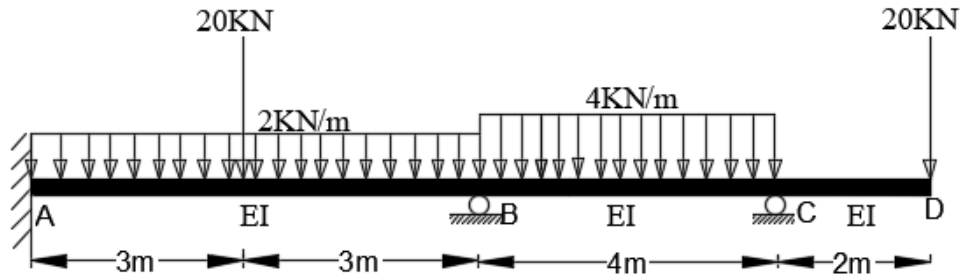
QUESTION FOUR (25 Marks)

Calculate the support reactions and moment in the continuous beam shown in Fig. below using the method of consistent deformations - Force Method; the flexural rigidity, EI , of the beam is constant throughout. **(25 marks)**



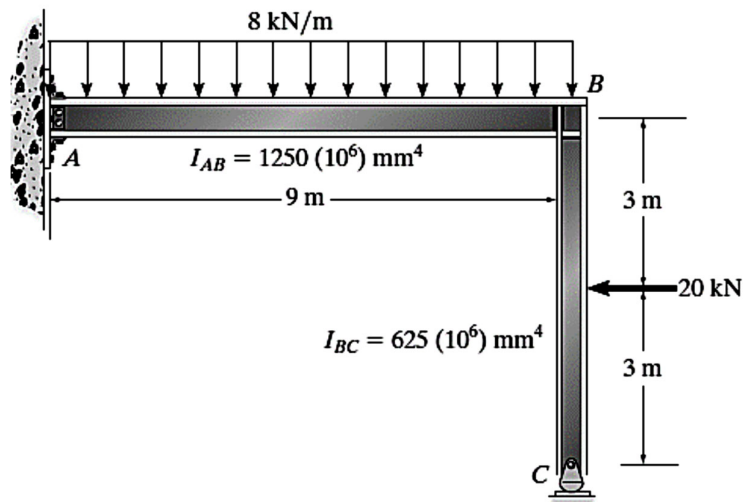
QUESTION FIVE (25 Marks)

Determine the moments and reactions for the continuous beam ABCD shown in Fig. below by slope deflection method and draw the shear force and bending moment diagrams. EI is constant. **(25 marks)**



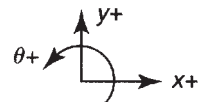
QUESTION SIX (25 Marks)

Using the *Moment distribution method*, determine the end moments and reactions, then draw the bending moment and shear diagrams for the beam shown in Fig. below. Support A is fixed and C is a pin. Given that, $E = 2 \times 10^5 \text{ N/mm}^2$ (25 marks)

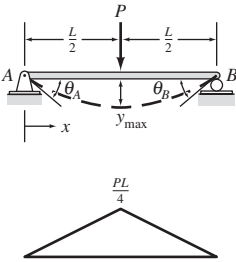
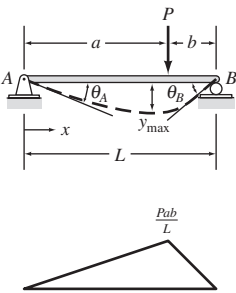
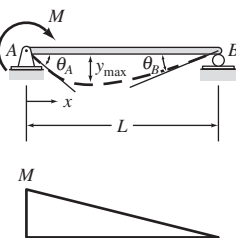


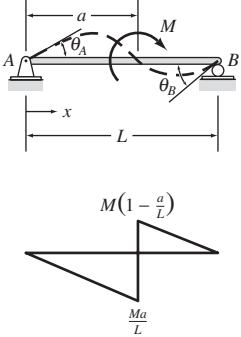
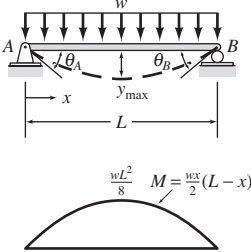
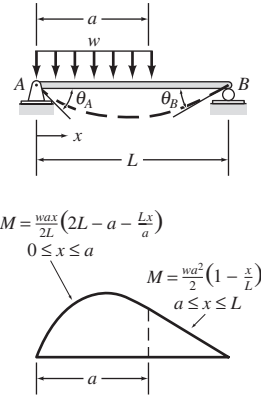
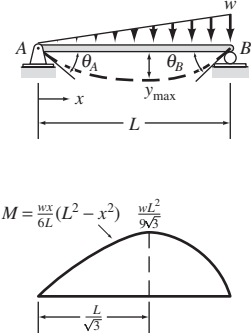
***** GOOD LUCK *****

BENDING MOMENTS, SLOPES, AND DEFLECTIONS OF BEAMS UNDER VARIOUS LOADING CONDITIONS



Beam, Loading, and Bending Moment Diagram	Equations for Slope and Deflection
	$0 \leq x \leq a :$ $\theta = \frac{P}{2EI}(x^2 - 2ax)$ $y = \frac{P}{6EI}(x^3 - 3ax^2)$ $a \leq x \leq L :$ $\theta = -\frac{Pa^2}{2EI}$ $y = \frac{Pa^2}{6EI}(a - 3x)$ $\theta_B = -\frac{Pa^2}{2EI}; \quad y_B = -\frac{Pa^2}{6EI}(3L - a)$
	$0 \leq x \leq a :$ $\theta = -\frac{Mx}{EI}$ $y = -\frac{Mx^2}{2EI}$ $a \leq x \leq L :$ $\theta = -\frac{Ma}{EI}$ $y = \frac{Ma}{2EI}(a - 2x)$ $\theta_B = -\frac{Ma}{EI}; \quad y_B = -\frac{Ma}{2EI}(2L - a)$
	$0 \leq x \leq a :$ $\theta = \frac{w}{6EI}(3ax^2 - 3a^2x - x^3)$ $y = \frac{w}{24EI}(4ax^3 - 6a^2x^2 - x^4)$ $a \leq x \leq L :$ $\theta = -\frac{wa^3}{6EI}$ $y = \frac{wa^3}{24EI}(a - 4x)$ $\theta_B = -\frac{wa^3}{6EI}; \quad y_B = -\frac{wa^3}{24EI}(4L - a)$
	$0 \leq x \leq a :$ $\theta = \frac{w}{24EIa}(x^4 - 4ax^3 + 6a^2x^2 - 4a^3x)$ $y = \frac{w}{120EIa}(x^5 - 5ax^4 + 10a^2x^3 - 10a^3x^2)$ $a \leq x \leq L :$ $\theta = -\frac{wa^3}{24EI}$ $y = \frac{wa^3}{120EI}(-5x + a)$ $\theta_B = -\frac{wa^3}{24EI}; \quad y_B = -\frac{wa^3}{120EI}(5L - a)$

Beam, Loading, and Bending Moment Diagram	Equations for Slope and Deflection
	$0 \leq x \leq \frac{L}{2} :$ $\theta = \frac{P}{16EI} (4x^2 - L^2)$ $y = \frac{P}{48EI} (4x^3 - 3L^2x)$ $\theta_A = -\frac{PL^2}{16EI}; \quad \theta_B = \frac{PL^2}{16EI}$ $y_{\max} = -\frac{PL^3}{48EI}$
	$0 \leq x \leq a :$ $\theta = \frac{Pb}{6EIL} (3x^2 + b^2 - L^2)$ $y = \frac{Pb}{6EIL} (x^3 + b^2x - L^2x)$ $a \leq x \leq L :$ $\theta = \frac{Pa}{6EIL} [L^2 - a^2 - 3(L - x)^2]$ $y = \frac{Pa(L-x)}{6EIL} (x^2 + a^2 - 2Lx)$ $\theta_A = -\frac{Pb}{6EIL} (L^2 - b^2)$ $\theta_B = \frac{Pa}{6EIL} (L^2 - a^2)$ <p>For $a \geq b :$</p> $y_{\max} = -\frac{Pb}{9\sqrt{3}EIL} (L^2 - b^2)^{3/2}$ $\text{at } x = \left(\frac{L^2 - b^2}{3} \right)^{1/2}$
	$\theta = -\frac{M}{6EIL} (3x^2 - 6Lx + 2L^2)$ $y = -\frac{M}{6EIL} (x^3 - 3Lx^2 + 2L^2x)$ $\theta_A = -\frac{ML}{3EI}; \quad \theta_B = \frac{ML}{6EI}$ $y_{\max} = -\frac{ML^2}{9\sqrt{3}EI}$ $\text{at } x = L \left(1 - \frac{1}{\sqrt{3}} \right)$

Beam, Loading, and Bending Moment Diagram	Equations for Slope and Deflection
	$0 \leq x \leq a :$ $\theta = \frac{M}{6EI L} (-3x^2 + 6aL - 3a^2 - 2L^2)$ $y = \frac{M}{6EI L} (-x^3 + 6aLx - 3a^2x - 2L^2x)$ $\theta_A = \frac{M}{6EI L} (6aL - 3a^2 - 2L^2)$ $\theta_B = \frac{M}{6EI L} (L^2 - 3a^2)$
	$\theta = -\frac{w}{24EI} (4x^3 - 6Lx^2 + L^3)$ $y = -\frac{w}{24EI} (x^4 - 2Lx^3 + L^3x)$ $\theta_A = -\frac{wL^3}{24EI}$ $\theta_B = \frac{wL^3}{24EI}$ $y_{\max} = -\frac{5wL^4}{384EI} \text{ at } x = \frac{L}{2}$
	$0 \leq x \leq a :$ $\theta = -\frac{w}{24EI L} [4Lx^3 - 6a(2L - a)x^2 + a^2(2L - a)^2]$ $y = -\frac{w}{24EI L} [Lx^4 - 2a(2L - a)x^3 + a^2(2L - a)^2x]$ $a \leq x \leq L :$ $\theta = -\frac{wa^2}{24EI L} (6x^2 - 12Lx + a^2 + 4L^2)$ $y = -\frac{wa^2}{24EI L} (L - x)(-2x^2 + 4Lx - a^2)$ $\theta_A = -\frac{wa^2}{24EI L} (2L - a)^2$ $\theta_B = \frac{wa^2}{24EI L} (2L^2 - a^2)$
	$\theta = -\frac{w}{360EI L} (15x^4 - 30L^2x^2 + 7L^4)$ $y = -\frac{w}{360EI L} (3x^5 - 10L^2x^3 + 7L^4x)$ $\theta_A = -\frac{7wL^3}{360EI}$ $\theta_B = \frac{wL^3}{45EI}$ $y_{\max} = -0.00652 \frac{wL^4}{EI} \text{ at } x = 0.5193L$

Fixed End Moments

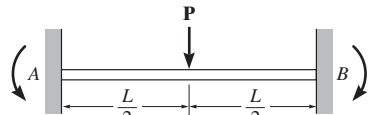


Diagram: A beam of length L fixed at both ends A and B . A point load P is applied at the center, dividing the beam into two equal segments of length $\frac{L}{2}$.

$$(FEM)_{AB} = \frac{PL}{8} \quad (FEM)_{BA} = \frac{PL}{8}$$

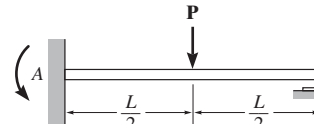


Diagram: A beam of length L fixed at end A and pinned at end B . A point load P is applied at the center, dividing the beam into two equal segments of length $\frac{L}{2}$.

$$(FEM)'_{AB} = \frac{3PL}{16}$$

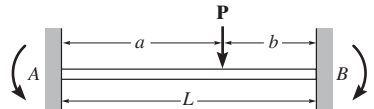


Diagram: A beam of length L fixed at both ends A and B . A point load P is applied at a distance a from end A and b from end B , where $L = a + b$.

$$(FEM)_{AB} = \frac{Pb^2a}{L^2} \quad (FEM)_{BA} = \frac{Pa^2b}{L^2}$$

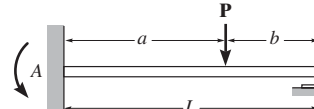


Diagram: A beam of length L fixed at end A and pinned at end B . A point load P is applied at a distance a from end A and b from end B , where $L = a + b$.

$$(FEM)'_{AB} = \left(\frac{P}{L^2}\right)(b^2a + \frac{a^2b}{2})$$

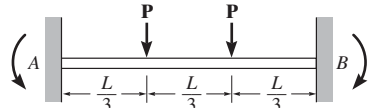


Diagram: A beam of length L fixed at both ends A and B . Three equal point loads P are applied, dividing the beam into four equal segments of length $\frac{L}{3}$.

$$(FEM)_{AB} = \frac{2PL}{9} \quad (FEM)_{BA} = \frac{2PL}{9}$$

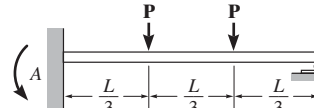


Diagram: A beam of length L fixed at end A and pinned at end B . Three equal point loads P are applied, dividing the beam into four equal segments of length $\frac{L}{3}$.

$$(FEM)'_{AB} = \frac{PL}{3}$$

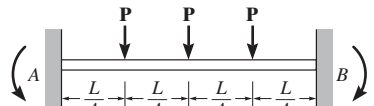


Diagram: A beam of length L fixed at both ends A and B . Four equal point loads P are applied, dividing the beam into five equal segments of length $\frac{L}{4}$.

$$(FEM)_{AB} = \frac{5PL}{16} \quad (FEM)_{BA} = \frac{5PL}{16}$$

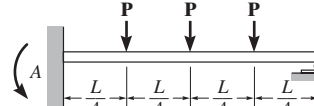


Diagram: A beam of length L fixed at end A and pinned at end B . Four equal point loads P are applied, dividing the beam into five equal segments of length $\frac{L}{4}$.

$$(FEM)'_{AB} = \frac{45PL}{96}$$

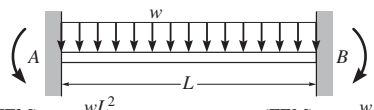


Diagram: A beam of length L fixed at both ends A and B . A uniformly distributed load w is applied downwards along the entire length of the beam.

$$(FEM)_{AB} = \frac{wL^2}{12} \quad (FEM)_{BA} = \frac{wL^2}{12}$$

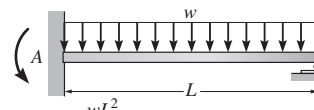


Diagram: A beam of length L fixed at end A and pinned at end B . A uniformly distributed load w is applied downwards along the entire length of the beam.

$$(FEM)'_{AB} = \frac{wL^2}{8}$$

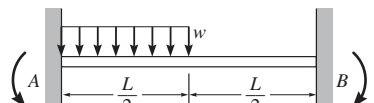


Diagram: A beam of length L fixed at both ends A and B . A triangularly distributed load w is applied, with the maximum intensity w at the left end A and zero at the right end B .

$$(FEM)_{AB} = \frac{11wL^2}{192} \quad (FEM)_{BA} = \frac{5wL^2}{192}$$

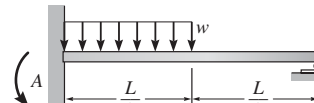


Diagram: A beam of length L fixed at end A and pinned at end B . A triangularly distributed load w is applied, with the maximum intensity w at the left end A and zero at the right end B .

$$(FEM)'_{AB} = \frac{9wL^2}{128}$$

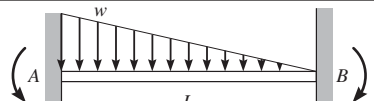


Diagram: A beam of length L fixed at both ends A and B . A triangularly distributed load w is applied, with the maximum intensity w at the right end B and zero at the left end A .

$$(FEM)_{AB} = \frac{wL^2}{20} \quad (FEM)_{BA} = \frac{wL^2}{30}$$

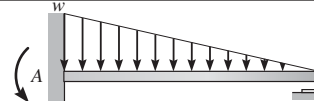


Diagram: A beam of length L fixed at end A and pinned at end B . A triangularly distributed load w is applied, with the maximum intensity w at the right end B and zero at the left end A .

$$(FEM)'_{AB} = \frac{wL^2}{15}$$

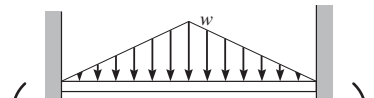


Diagram: A beam of length L fixed at both ends A and B . A triangularly distributed load w is applied, with the maximum intensity w at the center of the beam.

$$(FEM)_{AB} = \frac{5wL^2}{96} \quad (FEM)_{BA} = \frac{5wL^2}{96}$$

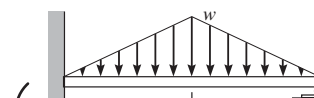


Diagram: A beam of length L fixed at end A and pinned at end B . A triangularly distributed load w is applied, with the maximum intensity w at the center of the beam.

$$(FEM)'_{AB} = \frac{5wL^2}{64}$$

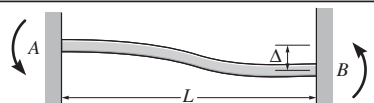


Diagram: A beam of length L fixed at both ends A and B . A vertical displacement Δ is applied at end B .

$$(FEM)_{AB} = \frac{6EI\Delta}{L^2} \quad (FEM)_{BA} = \frac{6EI\Delta}{L^2}$$




Diagram: A beam of length L fixed at end A and pinned at end B . A vertical displacement Δ is applied at end B .

$$(FEM)'_{AB} = \frac{3EI\Delta}{L^2}$$